



Design of Water Transmission System With Pumping

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Abstract:

As population increases, city grows extensively which leads to increase in demand of water for domestic, irrigation and industries. Also as the lifestyle of society is changing, they are getting more concerned about hygiene. So, it is duty of authorities to find nearest source of water, treat, store and use it optimally. This requires well designed water transmission and distribution system for providing treated water to consumers. Present study is related to the water transmission network *i.e.* flow of water from water treatment plant to elevated service reservoir. Main objective of this study was to determine optimum cost of the network. Here, the four reservoir system was considered with low head of source reservoir and higher heads of receiving reservoirs. Dynamic program was used for optimal design of water transmission network involving pumping using the Hazen-Williams equation. Here, the diameter equation was used for assuming initial value of diameter for pipes. Various values of diameter (on higher and lower side of calculated diameter) at an interval of 25 mm were considered and its effect on overall cost of network was analyzed. By using dynamic programming, the set of diameters which gave minimum cost of the network was considered as the optimal solution. Design process helps to identify head of service reservoir and diameters of pipes. The necessary cost data was taken from Maharashtra Jeevan Pradhikaran, Amravati for the year 2019-20.

Keywords: Dynamic programming, Hazen-Williams equation, Optimum cost, Optimal design, Four reservoir system, Transmission network.

1. Introduction:

As population is increasing demand of water for domestic, irrigation and industrial purpose has been increased. Also due to education and up gradation in lifestyle of society, they are getting more concerned about hygiene so, its duty of authorities to provide treated water to consumers. For this, they have to search nearby source of water, treat it and supply it according to the necessity. Water sources like lakes or rivers are not able to fulfill current water demand of society it may be due to various reasons like

increase in population, global warming, variation in rainfall, climatic changes etc. so optimal utilization of it is on the upmost priority list. Every year Water Resource Department (WRD) spends ample of money on water distribution project, but some minute saving of cost by improving design results in acceptable saving in total cost of project. Also improvement in design helps in optimal utilization of water.

Water, air and food are the basic needs of human for survival. Water supply network (WSN) is the most important part

in urbanization and every WSN has its unique design depending upon environmental, geological and economical factors. Following are the components for WSN:

- 1) Water source and intake well;
- 2) Water treatment plant and storage tank;
- 3) Water transmission network; and
- 4) Water distribution network.

Water from the source is transferred to water treatment plant (WTP). At WTP, required treatment is given to the water depending upon quality of water. Then treated water is stored in storage tank also called as main balance reservoir (MBR). From MBR, water is transferred to elevated service reservoir (ESR) this is called as water transmission network and from ESR water is proceed to consumer this is called as water distribution network.

Depending upon the level of the source and that of the city, topography of the area and other local considerations, the water may flow into the transmission network in the following three ways:

- 1) By gravitational system;
- 2) By pumping system; and
- 3) Combination of both gravity and pumping system.

This paper emphasizes on combination of both gravity and pumping system. Flow path algorithm and Hazen-Williams equation was developed for optimal design of three reservoir system.

2. Hydraulic Modeling Equations and Equations Used for Designing:

2.1 Economical diameter for pumping main:

For optimal conditions, we must choose such a diameter, which together with the pumping cost, will make the total

annual expenses, the minimum. The diameter which provides such optimal conditions is known as the economical diameter of the pipe (Garg, 2014).

$$D = (0.97 \text{ to } 1.22)\sqrt{\bar{Q}} \quad (1)$$

2.2 Pumping main diameter equation:

While pumping water from low head to higher head in pipe it creates pressure head difference by the use of pump which is also known as rising main and for this the diameter can be calculated by using following equation.

$$D = \left(\frac{51.98\gamma LQ^{1.852}}{mC_H^{1.852}} \frac{K_t}{K_m} \right)^{\frac{1}{m+4.870}} \quad (2)$$

2.3 Hazen Williams formula:

Hazen-Williams equation is empirically and dimensionally non-homogeneous equation and was originally introduced in 1902. It is the most widely used for design of pressure main. This formula is applicable for smooth turbulent flow.

$$V = 0.849C_H R^{0.63} S^{0.54} \quad (3)$$

$$h_f = \frac{10.674LQ^{1.852}}{C_H^{1.852} D^{4.870}} \quad (4)$$

3. Cost Function:

For computing cost of transmission network it is important to identify all the components which are involved in it and whose slight change in property shows considerable change in the cost of network. Major components of transmission network which affect the cost of network are as follows:

1. Cost of Pipe;
2. Cost of Pump; and
3. Cost of Energy.

3.1 Cost of Pipe:

Generally, various pipes are used for conveying water from source to its

destination. Cast iron or ductile iron pipes are most popular in all as they give resistance to corrosion and are more durable.

$$C_m = K_m L_i D_i^m \quad (5)$$

Where,

K_m and m = Cost parameter for pipes;

L_i = Length of pipe, m; and

D_i = Diameter of pipe, m.

CI pipe was considered for water transmission from source to elevated service reservoirs (ESR). To determine cost of pipe, schedule rate of pipe per meter length was taken from Maharashtra Jeevan Pradhikaran (MJP), Amravati. Total Capitalized Cost of Pipe v/s Diameter of Pipe is shown in Fig. 1. According to Fig. 1 cost parameters for CI pipe are $K_m = 35242$, $m = 1.403$ and $R^2 = 0.995$.

3.2 Cost of Pump:

Pumping is involved to transfer water from low head source; it can be lake or river to the high head ESR. Pumping plant and pumping house cost is included in pumping system. The cost function for pumping calculates the power of pump which is proportional to cost of pump house.

$$C_p = (1 + S)K_p P \quad (6)$$

Where,

K_p = Cost parameter of pump; and

P = Power of pump, KW; and

S = Standby Pump.

$$P = \frac{\gamma Q h_0}{1000\eta} \quad (7)$$

Where,

η = Efficiency of pump;

γ = Mass density of fluid;

Q = Discharge, m³/s; and

h_0 = Pumping head, m.

To determine cost of pump, schedule data was taken from MJP, Amravati. Cost of Pump v/s Diameter of Pipe is shown in Fig. 2. Cost parameter for pump from Fig. 2 are $K_p = 17695$ and $R^2 = 0.978$

3.3 Cost of Energy:

Unlike cost of pipe and pump it is not one time investment, rather it is an annual expenditure and with huge considerable cost. This cost is proportional to the power consume, indirectly it is proportional to discharge and pumping head, h_0 .

$$C_e = \frac{8760 P F_A F_D R_e}{r} \quad (8)$$

Where,

D = Diameter of pipe, m;

F_A = Annual averaging factor;

F_D = Daily averaging factor for the discharge;

R_e = Electricity rate; and

R = Rate of interest.

4. Design Example:

Fig. 3 shows basic example of WTN which consists of four reservoirs 3, 3', 2', 1 and two junctions $J1$ and $J2$. Corresponding data of reservoirs and pipe is as follows:

Table 1: Data of Three Reservoir System

| Pipe/Reservoir | H | L | Q |
|----------------|-----|------|------|
| S | 20 | 300 | 0.6 |
| 3 | | 5000 | 0.6 |
| 3' | 85 | 3000 | 0.2 |
| 2 | | 4000 | 0.4 |
| 2' | 75 | 2000 | 0.15 |
| 1 | 60 | 4500 | 0.25 |

5. Procedure of Dynamic Programming and Cost Calculation:

Trial 1

Stage 1:

1. Diameter of pipe 3 (D_3), 3' ($D_{3'}$) and 2' ($D_{2'}$) was calculated by using eq. 1 and diameter of pipe 1 (D_1) and 2 (D_2) was calculated by eq. 4. D_5 was calculated by raising main equation i.e. eq. 2.
2. Diameter D_3 and $D_{3'}$ was kept fix and $D_{2'}$ was varied in the interval of 25mm on higher and lower side of calculated values. D_1 and D_2 value was changing with variation in $D_{2'}$ values and it was rounded up to the next higher value.

Stage 2:

1. From stage 1, diameter $D_{2'}$ and D_1 was selected for which total cost of network was minimum. Selected diameter $D_{2'}$ and D_1 also D_3 was kept fix while $D_{3'}$ was varied in the interval of 25mm on higher and lower side of calculated values. D_2 value was changing with variation in $D_{3'}$ values and it was rounded up to the next higher value.

Stage 3:

1. From stage 2, diameter $D_{3'}$ was selected which shows global minimum value of total cost of network. Selected diameters of $D_{3'}$, $D_{2'}$ and D_1 was kept fix while D_3 was varied in the interval of 25mm on higher and lower side of calculated value.

Trial 2

Stage 1:

1. Selected diameters of D_3 and $D_{3'}$ from trial 1 was kept fix and $D_{2'}$ was varied with the interval of 25mm on higher and lower side of selected value from previous trial. D_1 and D_2 value was changing with variation in $D_{2'}$ values and it was rounded up to the next higher value.

Stage 2:

1. From previous stage, diameter $D_{2'}$ and D_1 was selected for which total cost of network was minimum. Selected diameter $D_{2'}$, D_1 and D_3 was kept fix while $D_{3'}$ was varied with the interval of 25mm on higher and lower side of selected value from previous trial. D_2 value was changing with variation in $D_{3'}$ values and it was rounded up to the next higher value.

Stage 3:

1. From stage 2, diameter $D_{3'}$ was selected which have global minimum value of total cost of network. Selected diameter $D_{3'}$, $D_{2'}$ and D_1 was kept fix while D_3 was varied with the interval of 25mm on higher and lower side of selected value from previous trial.
2. In this stage same set of diameters were giving global minimum value of total cost of network. So, no further iterations required. If there is change in diameter set and minimum cost value same steps are followed in further trials.

Results and Discussion:

Trial 1:

Stage 1:

From Table 3 and Fig. 4 it was concluded that 775mm dia. of D_3 , 450mm dia. of $D_{3'}$, 575mm dia. of D_2 , 425mm dia. of $D_{2'}$ and 475mm dia. of D_1 gave global minimum value of total cost of network. In stage 2 above values of $D_{2'}$ and D_1 was kept fix.

Stage 2:

From Table 4 and Fig. 5 it was concluded that 775mm dia. of D_3 , 525mm dia. of $D_{3'}$, 625mm dia. of D_2 , 425mm dia. of $D_{2'}$ and 475mm dia. of D_1 gave global minimum value of total cost of network. In

stage 3, above value of D_3 , D_2 and D_1 was kept fix.

Stage 3:

From Table 5 and Fig. 6 it was concluded that 800mm dia. of D_3 , 525mm dia. of D_3 , 625mm dia. of D_2 , 425mm dia. of D_2 and 475mm dia. of D_1 gave global minimum value of total cost of network.

Trial 2:

Stage 1:

In this stage D_3 and D_3 selected from trial 1 was kept fix. From Table 6 and Fig. 7 it was concluded that 800mm dia. of D_3 , 525mm dia. of D_3 , 600mm dia. of D_2 , 475mm dia. of D_2 and 475mm dia. of D_1 gave global minimum value of total cost of network. In stage 2 above values of D_2 and D_1 was kept fix.

Stage 2:

From Table 7 and Fig. 8 it was concluded that 800mm dia. of D_3 , 525mm dia. of D_3 , 600mm dia. of D_2 , 475mm dia. of D_2 and 475mm dia. of D_1 gave global minimum value of total cost of network. In stage 3, above value of D_3 , D_2 and D_1 was kept fix.

Stage 3:

From Table 8 and Fig. 9 it was concluded that 800mm dia. of D_3 , 525mm dia. of D_3 , 600mm dia. of D_2 , 475mm dia. of D_2 and 475mm dia. of D_1 gave global minimum value of total cost of network. As in this stage with the variation in dia. of pipe 3 there is no change in minimum cost so no further trials required for analysis.

5. Conclusions

This dynamic program was developed to analyse water transmission network using pumping. In this dynamic programme analysis starts from end node and it minimizes total cost of network by varying diameters of pipe in various trials. Here minimum cost was obtained in stage

3 of trial 2 with no change in set of diameters, so no need of further analysis. It was also useful for predicting pumping head from source to service reservoir. This approach can be utilized in future for complex transmission network.

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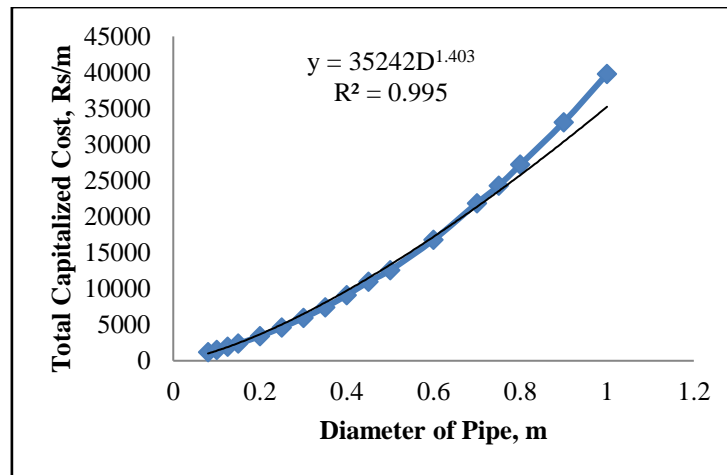


Fig. 1: Total Capitalized Cost v/s Diameter of Pipe

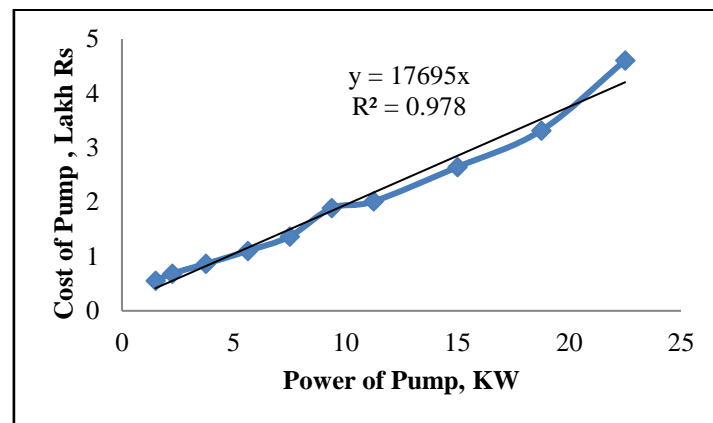


Fig. 2: Capitalized Cost of Pump v/s Power of Pump

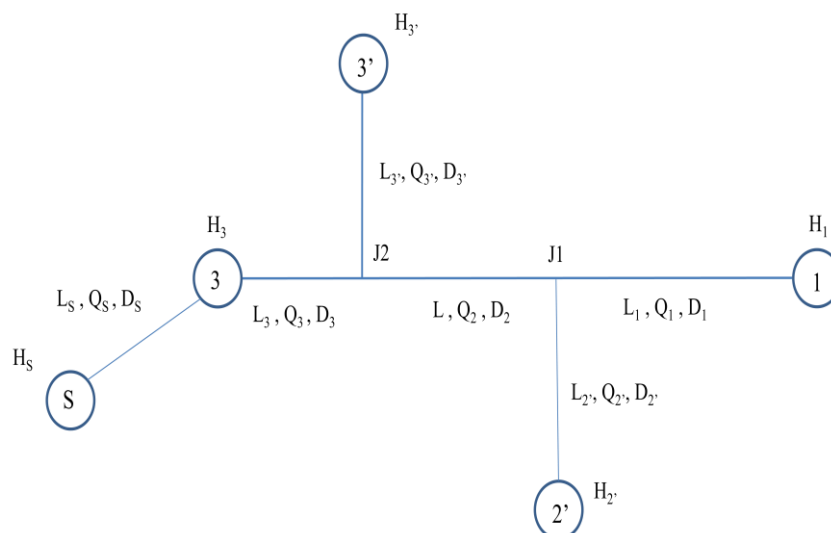


Fig. 3: Four Reservoir System using Pumping

Table 2: Initially Assumed Diameter Values

| D_3 (mm) | $D_{3'}$ (mm) | D_2 (mm) | $D_{2'}$ (mm) | D_1 (mm) | Total cost of network (Lakh, Rs) |
|---------------|------------------|---------------|------------------|---------------|-------------------------------------|
| 775 | 450 | 600 | 400 | 475 | 7519.128 |

Table 3: Results Analysis of Trial 1-Stage 1

| Sr. No. | D_3 (mm) | $D_{3'}$ (mm) | D_2 (mm) | $D_{2'}$ (mm) | D_1 (mm) | D_S (mm) | H_3 (m) | Total cost of network (Lakh, Rs) |
|---------|------------|---------------|------------|---------------|------------|------------|-----------|----------------------------------|
| 1 | 775 | 450 | 675 | 350 | 450 | 825 | 103.376 | 7568.772 |
| 2 | 775 | 450 | 625 | 375 | 450 | 825 | 103.376 | 7502.701 |
| 3 | 775 | 450 | 600 | 400 | 475 | 825 | 103.376 | 7519.128 |
| 4 | 775 | 450 | 575 | 425 | 475 | 825 | 103.376 | 7496.525 |
| 5 | 775 | 450 | 575 | 450 | 475 | 825 | 103.376 | 7514.242 |
| 6 | 775 | 450 | 575 | 475 | 475 | 825 | 103.376 | 7532.360 |
| 7 | 775 | 450 | 575 | 500 | 475 | 825 | 103.376 | 7550.867 |

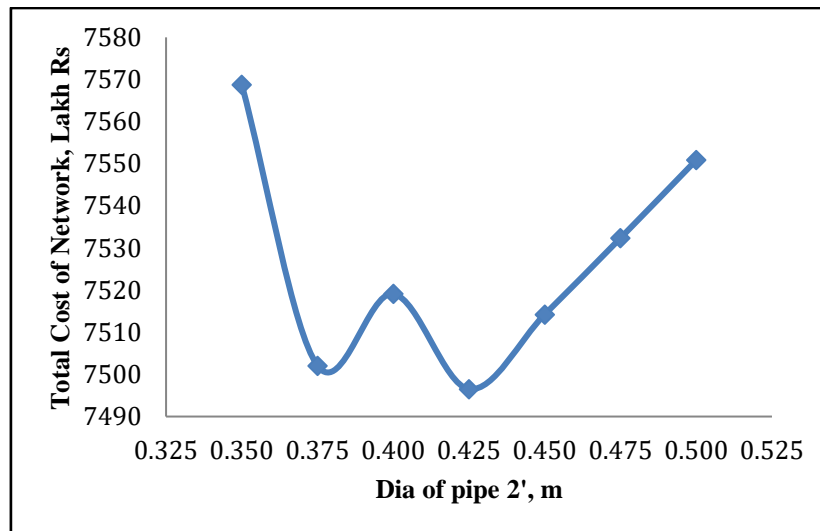


Fig. 4: Total Cost of Network v/s Dia. of Pipe 2'

Table 4: Results Analysis of Trial 1-Stage 2

| Sr. No. | D_3 (mm) | $D_{3'}$ (mm) | D_2 (mm) | $D_{2'}$ (mm) | D_1 (mm) | D_S (mm) | H_3 (m) | Total cost of network (Lakh, Rs) |
|---------|------------|---------------|------------|---------------|------------|------------|-----------|----------------------------------|
| 1 | 775 | 400 | 525 | 425 | 475 | 825 | 110.856 | 7747.334 |
| 2 | 775 | 425 | 550 | 425 | 475 | 825 | 106.475 | 7588.620 |
| 3 | 775 | 450 | 575 | 425 | 475 | 825 | 103.376 | 7496.525 |
| 4 | 775 | 475 | 600 | 425 | 475 | 825 | 101.140 | 7449.736 |
| 5 | 775 | 500 | 625 | 425 | 475 | 825 | 99.5 | 7434.517 |
| 6 | 775 | 525 | 625 | 425 | 475 | 825 | 98.278 | 7400.566 |
| 7 | 775 | 550 | 650 | 425 | 475 | 825 | 97.354 | 7423.611 |
| 8 | 775 | 575 | 650 | 425 | 475 | 825 | 96.646 | 7416.985 |
| 9 | 775 | 600 | 675 | 425 | 475 | 825 | 96.099 | 7460.889 |

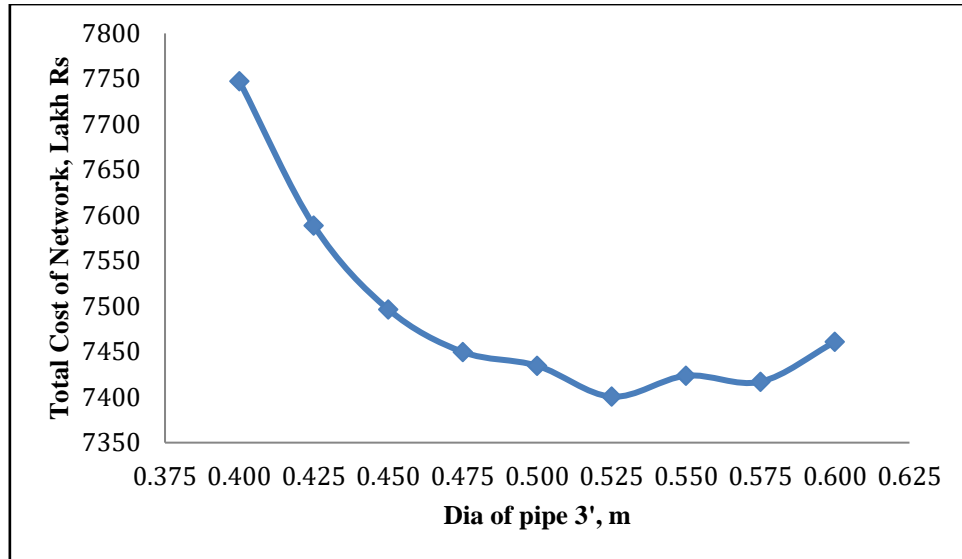


Fig. 5: Total Cost of Network v/s Dia. of Pipe 3'

Table 5: Results Analysis of Trial 1-Stage 3

| Sr. No. | D_3 (mm) | $D_{3'}$ (mm) | D_2 (mm) | $D_{2'}$ (mm) | D_1 (mm) | D_s (mm) | H_3 (m) | Total cost of network (Lakh, Rs) |
|---------|------------|---------------|------------|---------------|------------|------------|-----------|----------------------------------|
| 1 | 725 | 525 | 625 | 425 | 475 | 825 | 101.624 | 7460.952 |
| 2 | 750 | 525 | 625 | 425 | 475 | 825 | 99.788 | 7422.072 |
| 3 | 775 | 525 | 625 | 425 | 475 | 825 | 98.278 | 7400.566 |
| 4 | 800 | 525 | 625 | 425 | 475 | 825 | 97.029 | 7393.062 |
| 5 | 825 | 525 | 625 | 425 | 475 | 825 | 95.989 | 7396.941 |
| 6 | 850 | 525 | 625 | 425 | 475 | 825 | 95.119 | 7410.154 |
| 7 | 875 | 525 | 625 | 425 | 475 | 825 | 94.387 | 7431.086 |

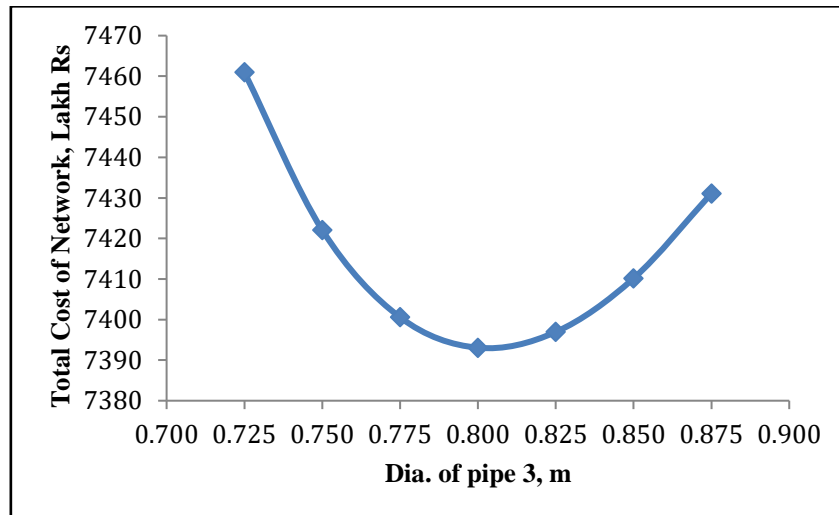


Fig. 6: Total Cost of Network v/s Dia. of Pipe 3

Table 6: Results Analysis of Trial 2-Stage 1

| Sr. No. | D_3 (mm) | $D_{3'}$ (mm) | D_2 (mm) | $D_{2'}$ (mm) | D_1 (mm) | D_S (mm) | H_3 (m) | Total cost of network (Lakh, Rs) |
|---------|------------|---------------|------------|---------------|------------|------------|-----------|----------------------------------|
| 1 | 800 | 525 | 725 | 375 | 450 | 825 | 97.029 | 7486.892 |
| 2 | 800 | 525 | 650 | 400 | 475 | 825 | 97.029 | 7417.001 |
| 3 | 800 | 525 | 625 | 425 | 475 | 825 | 97.029 | 7393.062 |
| 4 | 800 | 525 | 625 | 450 | 475 | 825 | 97.029 | 7410.779 |
| 5 | 800 | 525 | 600 | 475 | 475 | 825 | 97.029 | 7388.317 |
| 6 | 800 | 525 | 600 | 500 | 475 | 825 | 97.029 | 7406.824 |
| 7 | 800 | 525 | 600 | 525 | 475 | 825 | 97.029 | 7425.708 |

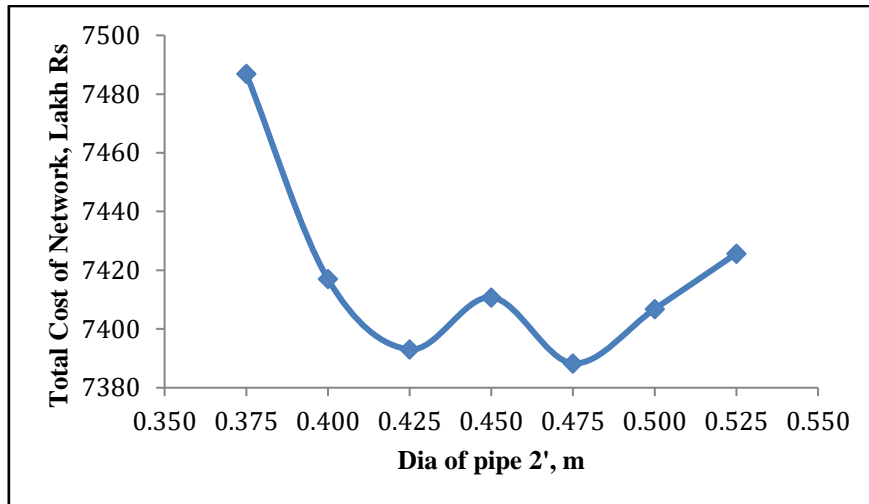


Fig. 7: Total Cost of Network v/s Dia. of Pipe 2'

Table 7: Results Analysis of Trial 2-Stage 2

| Sr. No. | D_3 (mm) | $D_{3'}$ (mm) | D_2 (mm) | $D_{2'}$ (mm) | D_1 (mm) | D_S (mm) | H_3 (m) | Total cost of network (Lakh, Rs) |
|---------|------------|---------------|------------|---------------|------------|------------|-----------|----------------------------------|
| 1 | 800 | 475 | 575 | 475 | 475 | 825 | 99.891 | 7438.163 |
| 2 | 800 | 500 | 600 | 475 | 475 | 825 | 98.251 | 7422.268 |
| 3 | 800 | 525 | 600 | 475 | 475 | 825 | 97.029 | 7388.317 |
| 4 | 800 | 550 | 625 | 475 | 475 | 825 | 96.105 | 7410.702 |
| 5 | 800 | 575 | 625 | 475 | 475 | 825 | 95.397 | 7404.076 |
| 6 | 800 | 600 | 625 | 475 | 475 | 825 | 94.849 | 7406.096 |

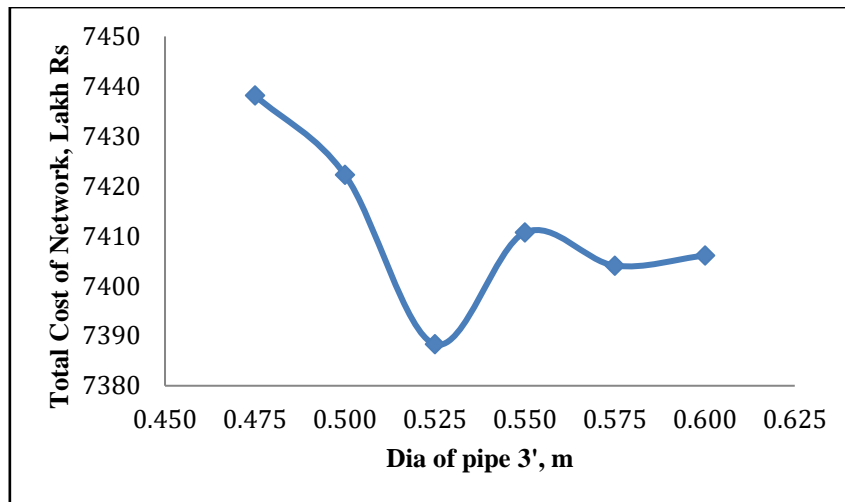


Fig. 8: Total Cost of Network v/s Dia. of Pipe 3'

Table 8: Results Analysis of Trial 2-Stage 3

| Sr. No. | D_3 (mm) | D_3' (mm) | D_2 (mm) | D_2' (mm) | D_1 (mm) | D_S (mm) | H_3 (m) | Total cost of network (Lakh, Rs) |
|---------|------------|-------------|------------|-------------|------------|------------|-----------|----------------------------------|
| 1 | 750 | 525 | 600 | 475 | 475 | 825 | 99.788 | 7417.327 |
| 2 | 775 | 525 | 600 | 475 | 475 | 825 | 98.278 | 7395.821 |
| 3 | 800 | 525 | 600 | 475 | 475 | 825 | 97.029 | 7388.317 |
| 4 | 825 | 525 | 600 | 475 | 475 | 825 | 95.989 | 7392.196 |
| 5 | 850 | 525 | 600 | 475 | 475 | 825 | 95.119 | 7405.409 |
| 6 | 875 | 525 | 600 | 475 | 475 | 825 | 94.387 | 7426.341 |

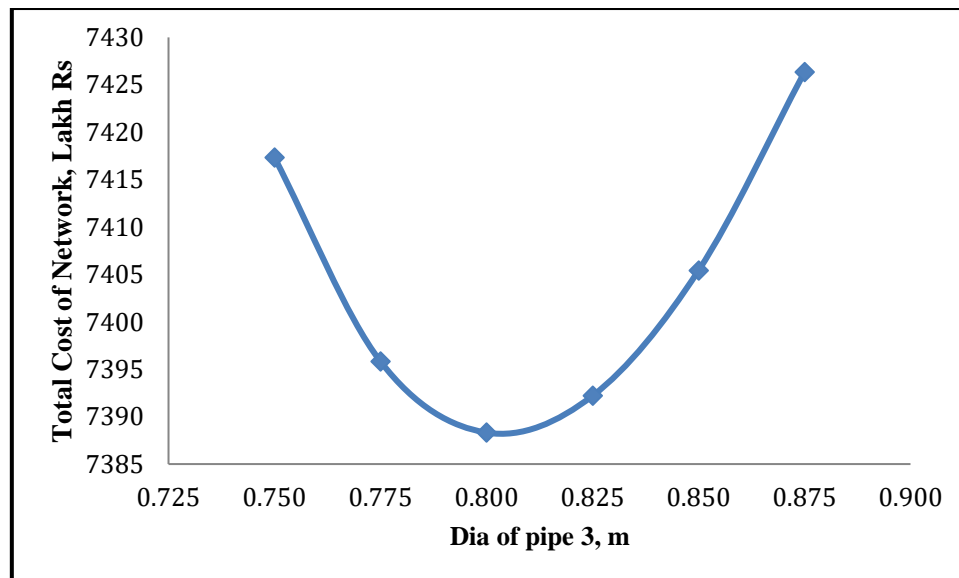


Fig. 9: Total Cost of Network v/s Dia. of Pipe 3'